

Development of SPT and DCPT Correlation for Sabkha Soils of Ras Al Khair in Eastern Saudi Arabia

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Abstract Standard Penetration Test (SPT) is commonly used to delineate subsurface strata conditions in routine geotechnical investigation campaign and provides quite useful information on the nature and type of the subsurface strata. Many geotechnical parameters of soils are associated with SPT and several SPT design correlations exist for foundation design. SPT is generally performed using automatic hammers equipped on truck mounted drilling rigs. Sabkha is a saline (generally coastal) deposit consisting of saturated, loose silty sand and clay. Sabkhas are not traversable for any kind of vehicle and drilling trucks often get stuck in sabkha areas, causing damage to the equipment. This limitation of vehicle movement restricts the acquisition of SPT data in sabkha areas which are prevalent in major parts of eastern Saudi Arabia. Dynamic Cone Penetration Test (DCPT) can be used as a good alternative to SPT in sabkha areas due to its portable tripod-based equipment. In this study, SPT and DCPT tests are carried out side by side on several sabkha sites and correlations are developed between SPT and DCPT data. The results indicate that a good correlation exists between both dynamic tests. The developed SPT and DCPT correlation can be effectively used to utilize well-established relationships between SPT and soil properties for sabkha deposits.

Keywords Standard Penetration Test, Dynamic cone penetration test, Sabkha soils, SPT-DCPT correlations

1. Introduction

During any standard geotechnical investigation program, a variety of field-testing methods are adopted, each with its own set of advantages and limitations. Among these, the Standard Penetration Test (SPT) is one of the most widely used techniques for identifying the strength, nature and classification of subsurface strata. Its popularity stems from the extensive database of empirical correlations that link SPT results with various geotechnical design parameters. These correlations make the SPT a valuable tool in estimating soil properties critical for design purposes. Dynamic Cone Penetration Test (DCPT) is another penetration-based field test applied, usually, for pavement investigations [21,24,27]. However, DCPT can also be used effectively as geotechnical investigation and foundation design tool [15,17,18]. The main advantages of DCPT over SPT are; (i) it is fast as no drilling operation is required, (iii) profiling is continuous, (iii) inexpensive, and (iv) particularly useful to investigate weak soil layers such as sabkhas. However, its major limitation is non retrieval of samples and limited penetration in very

dense and hard materials. Individually, each of these tests has its own limitations, but when used together, the SPT and DCPT can complement one another, providing a more comprehensive understanding of subsurface conditions that might not be fully uncovered by either test alone.

The coastal areas of Saudi Arabia, the east coast is surfaced with sabkha soils [3,5,19]. The sabkhas are identified by high concentrations of salt, shallow water tables and low shear strength [12]. As per Saudi Building Code [26], Sabkhas are salt bearing arid climate sediments. The sabkha sediments are highly variable in lateral and vertical extent; various soil types, primarily composed of clays, silts, fine sands, and organic matter are interlayered at random [7,9,10]. In general, sabkha sediments are characterized by high void ratios and low dry densities [1,2]. Accordingly, upon wetting sabkha soil is renowned for being highly compressible material with low bearing resistance, and hence considered among the poorest of foundation materials [1,2,6-11]. Sabkha areas are generally inaccessible to vehicles, and drilling rigs often become stuck, posing a risk of equipment damage. This restriction on vehicle movement creates significant challenges in conducting Standard Penetration Tests (SPT), limiting the availability and reliability of subsurface data in these regions. In sabkha regions, the Dynamic Cone Penetration Test (DCPT)

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Received: May 14, 2025; Accepted: Jun. 8, 2025; Published: Jun. 16, 2025

Published online at <http://journal.sapub.org/jce>

offers a viable alternative to the Standard Penetration Test (SPT), thanks to its portable, tripod-based equipment that allows for easier deployment in areas with limited accessibility.

DCPT has been correlated in literature with SPT as a supplementary test to reduce the number of SPT boreholes as well as for design of shallow foundations. SPT and DCPT correlations are generally used for subsurface strata strength classification, translation of SPT blow counts (N-values) into DCPT blow counts (N_{10} -values) and vice versa. The primary aim of converting one geotechnical parameter to another is to make effective use of the extensive correlation data available in the literature, which is typically developed through local experience, field measurements, design practices, and engineering preferences.

Several studies [13,14,16,22,23,28] have been performed to develop SPT-DCPT correlations in the past. Various efforts have also been made to utilize the Dynamic Cone Penetrometer Test (DCPT) in estimating the allowable bearing capacity of shallow foundations. [22] developed a correlation to assess the bearing capacity of shallow foundations for sandy soils in Ghana. In Tanzania, [23] established correlations between DCPT and SPT results for sandy soils. [20] also developed relationships between DCPT and SPT values to estimate the bearing capacities of foundations. However, all presented relationships cannot be used universally due to variations in different types of DCPT equipment worldwide and non-establishment of corrections related to hammer efficiency, overburden correction etc. to the DCPT procedures [28]. Therefore, it is essential to develop a correlation between SPT and DCPT for local sabkha soils.

In the eastern region of Saudi Arabia, especially along the coastal areas, the soils are predominantly calcareous and often exhibit cementation, accompanied by shallow groundwater levels [29]. As a result, examining the relationship between Standard Penetration Test (SPT) N-values and Dynamic Cone Penetration Test (DCPT) for these local soil conditions can be highly beneficial for any future construction in these areas. Such correlations can enhance the understanding of subsurface soil behavior and response as reflected in both

testing approaches.

2. Sites and Investigation Description

This study utilizes a dataset comprising Dynamic Cone Penetration Test (DCPT) soundings and Standard Penetration Test (SPT) profiles obtained from three sites of Ras Al Khair area (Site 1, Site 2, and Site 3) located in the eastern coastal region of Saudi Arabia. The locations of these sites are illustrated in Figure 1. Each site includes data from ten boreholes and ten corresponding DCPTs. The DCPTs were conducted at distances ranging from 3.0 to 5.0 meters from the SPT boreholes to maintain proximity while minimizing interference between tests.

A total of thirty (30) boreholes with SPTs were drilled at three different site locations using straight rotary drill rigs. Standard Penetration Tests (SPTs) were performed using automatic hammers at 0.50m depth intervals. These tests were performed generally in accordance with ASTM D-1586 using a split spoon sampler of 35mm inner dia. and 50mm outer diameter. DCPT soundings were also performed at thirty (30) locations in the vicinity of boreholes. DCPTs were performed using Super Heavy Dynamic Probe (DPSH-A) in accordance with BS EN ISO 22476-2 [15] using a hammer weighing 63.5 kg with a drop height of 50cm. The DCPT blow counts were recorded for each 10 cm penetration of cone.

Subsurface strata conditions

A particular feature, typical for the eastern coast of Saudi Arabia, is the flat and wide spreading coastal sabkha. Penetrating up to 10km inland, they cover areas of more than 100km². Sabkha is the Arabic term for flat salt-crust desert. The local terminology of the Gulf region describes the extensive, barren, salt encrusted, and periodically flooded coastal flats as well as inland salt flats. They consist basically of sand or finer unconsolidated substrate. Their surface is in equilibrium of aeolian deflation and aeolian sedimentation, controlled by the local shallow ground water level which is the lower limit of deflation.

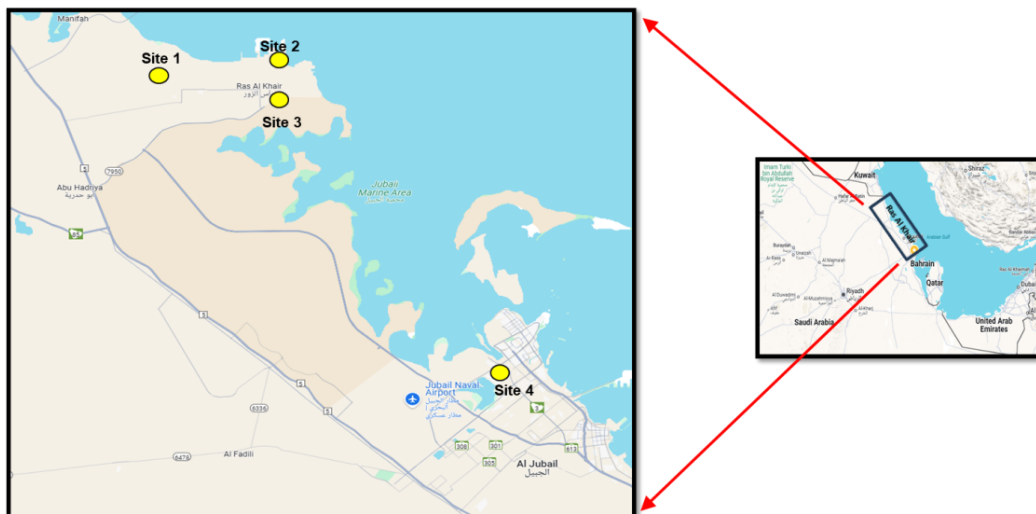


Figure 1. Vicinity Map showing locations of investigated sites

The chemical analysis results of Sabkha brine and the Arabian Gulf water are presented in Table 1. Based on the results, the Sabkha water is almost 3 to 6 times more concentrated than sea water in the vicinity. An X-ray diffraction analysis of Sabkha samples (Figure 2) shows that Halite (Sodium chloride) is present in significant amount in the sample and comprises about 15% of the sabkha matrix [7]. The other major cementing minerals in sabkha are Aragonite, Calcite and Gypsum.

Table 1. Chemical analysis of the sabkha brine and seawater in mg/mL (i.e. parts per thousand) [12]

Ions	Ras Al-Khair Brine	Seawater
Na ⁺	78.8	20.7
Mg ⁺⁺	10.32	2.30
K ⁺	3.06	0.73
Ca ⁺⁺	1.45	0.76
Fe ⁺⁺	Trace	Trace
Sr ⁺⁺	0.029	0.013
Cl ⁻	157.2	36.9
Br ⁻	0.49	0.121
(SO ₄) ⁻⁻	5.45	5.12
(HCO ₃) ⁻	0.087	0.128
PH	6.9	8.3
Conductivity*	208,000	46,200

*Microsiemens

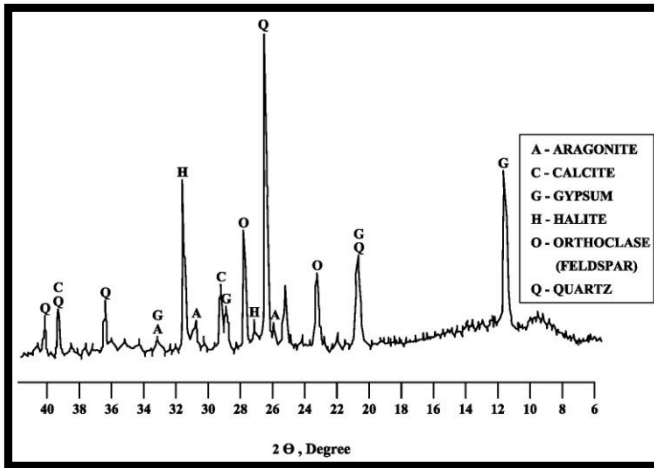


Figure 2. X-ray diffraction analysis of sabkha sample [8]

At the three (03) investigated sites, soil layers consisting of Poorly Graded SAND (SP) / Poorly Graded SAND with Silt (SP-SM) / Silty Sand (SM) have been encountered at all borehole locations in the study areas. The compactness of these sand layers is very loose to loose and therefore, these layers are classified as sabkha layers. Being very near to shoreline, the Sabkha layers found at the study sites are directly connected to sea water and exhibit characteristics of “coastal Sabkha”.

The groundwater table was encountered in the boreholes at shallower depths varying from 0.50 to 1.0m below natural

surface level. As the groundwater table is connected to the sea, it has very high dissolved salt content and may experience seasonal/tidal fluctuations.

3. SPT and DCPT Data Analysis

Standard Penetration Tests (SPTs) were conducted using automatic hammers at 0.50 meter depth intervals, extending to a maximum depth of 6.0 meters. The data obtained from these tests were used to evaluate the in-situ compactness or density of the subsurface materials. SPT blow counts for the final 30 cm penetration were referred as N_{60} values. For all boreholes across the three study sites, the recorded SPT- N values were normalized to N_{60} by applying energy and other relevant correction factors. Additionally, DCPT values (N_{10}) were extracted from averaging the DCPT soundings at depths corresponding to those of the SPTs against each 30 cm penetration.

From the thirty (30) SPT boreholes and corresponding DCPTs, approximately one hundred and fifty (150) data pairs of normalized SPT values (N_{60}) and DCPT values (N_{10}) were available for comparative analysis. However, to illustrate the correlation process, a typical subset of data points has been presented in Table 2.

Finally, from all these data points, N_{60} and corresponding N_{10} values were plotted and the graph is shown in Figure 3. It can be observed from the plot that a linear relationship of $y=0.4658x - 0.0807$ can be adopted for the local sabkha soils of eastern coast of Saudi Arabia.

Table 2. SPT and DCPT field data points

Depth (m)	Site 1		Site 2		Site 3	
	N_{60}	N_{10}	N_{60}	N_{10}	N_{60}	N_{10}
0.00	1	3	1	3	1	3
0.50	1	3	2	3	1	3
1.00	1	3	1	4	2	4
1.50	2	3	1	3	1	3
2.00	1	3	2	3	1	3
2.50	1	3	1	3	2	3
3.00	1	3	1	6	2	5
4.00	3	13	2	5	4	5
4.50	10	31	5	11	2	9

Furthermore, allowable bearing capacity values were also determined through both SPT and DCPT based methods using SPT and DCPT data sets for nine (09) different locations selected from three investigated sites. Burland & Burbidge method was used for calculating bearing capacities using the field standard penetration number, N_{60} . The methods proposed by [22], [4] and [25] were used for obtaining allowable bearing capacity values using DCPT data. The analysis was performed for a footing size of 1m x 1m, placed at a depth of 1.0m below grade level, The zone of influence in Burland and Burbidge method and DCPT based methods was taken either as 2B or to the bottom of loose soil layers

measured from the bottom of the foundation whichever was smaller. The N_{60} and N_{10} values were averaged over the zone of influence of foundations in the calculations. The results of allowable bearing capacity values using SPT and DCPT based methods are provided in Table 3.

Validation of proposed correlation

The correlation developed in this study can be applied to

sabkha soils of eastern region of Saudi Arabia. The validation of the correlation was performed using the SPT and DCPT data of sabkha soils of one of the sites in Jubail area (Site 4) which is about 100 Km away from Ras Al Khair area. The DCPT values predicted from the correlation were found to be in well agreement with the values actually obtained from the field tests (Figure 4).

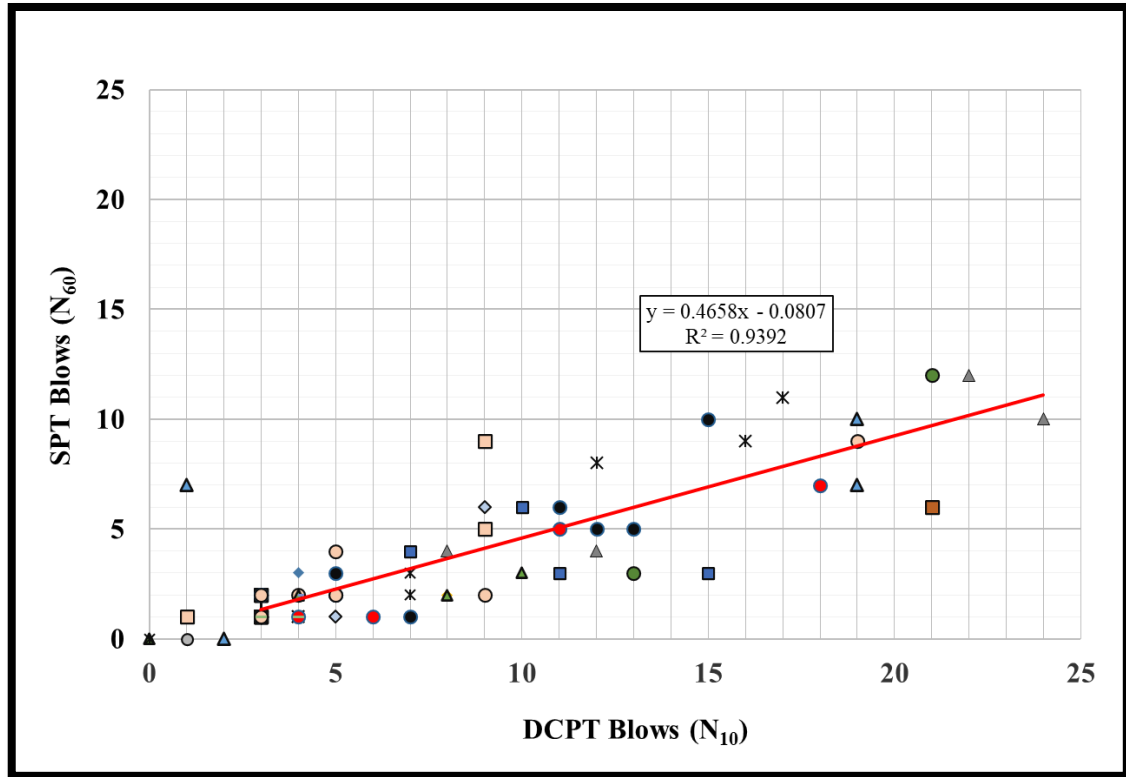


Figure 3. Plot of SPT and DCPT values for investigated sites

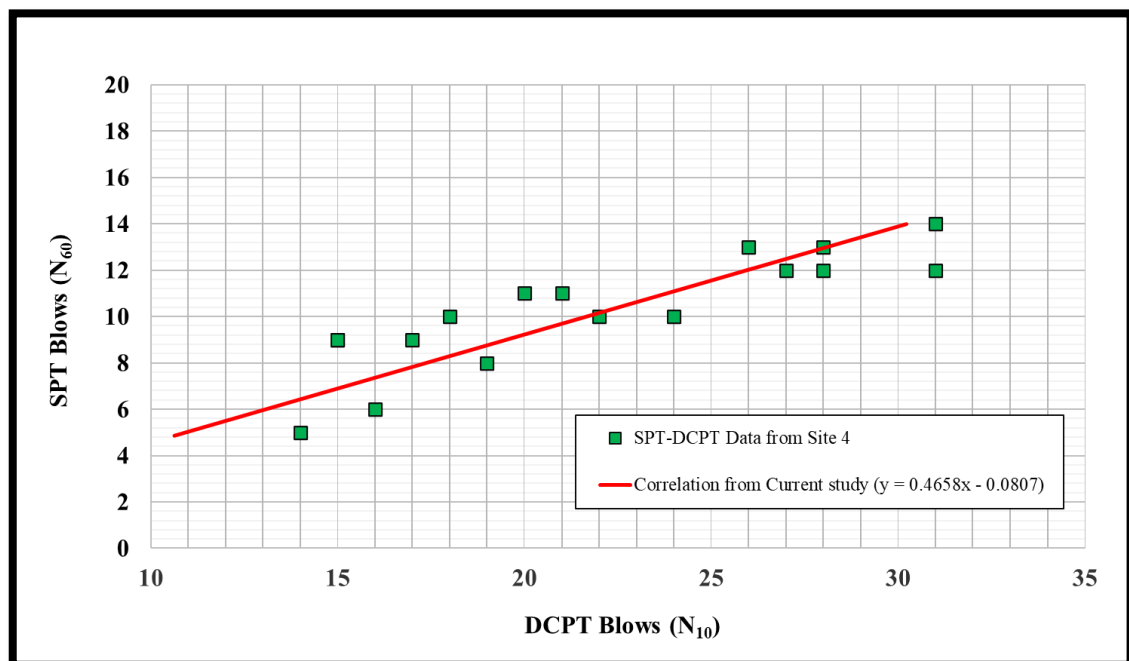
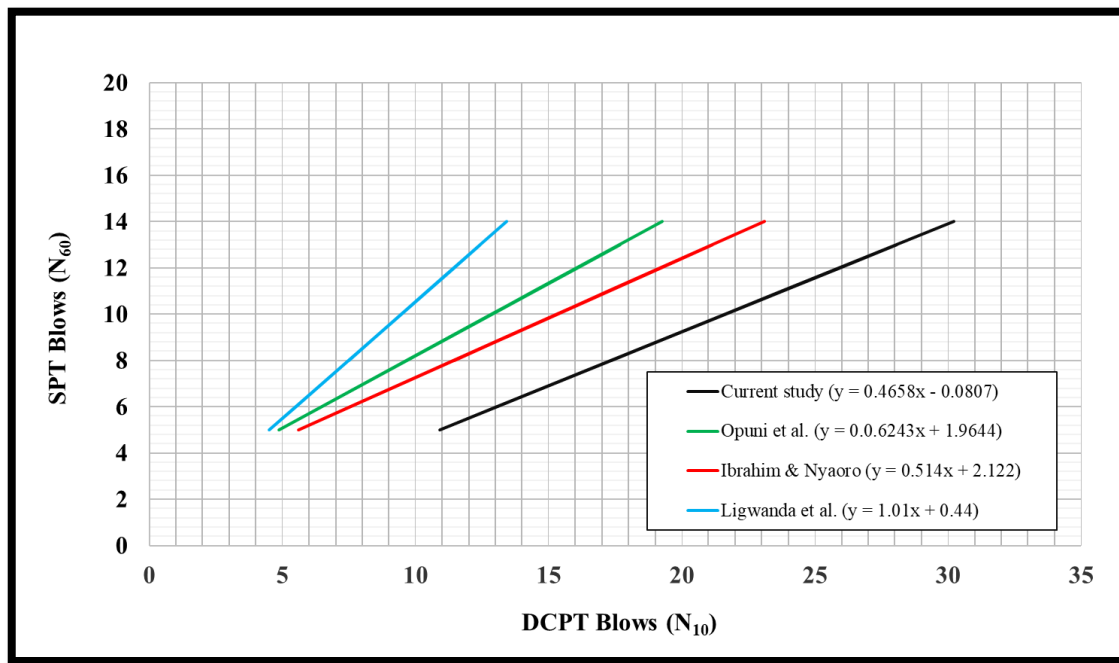


Figure 4. Validation of SPT and DCPT correlation using data of Site 4

Table 3. Allowable bearing capacities using SPT and DCPT Data

Case #	SPT Based Method Burland & Burbidge Method	DCPT Based Methods		
		[22]	[4]	[25]
		$q_a = 15.75N_{10} + 54.19$	$q_a = 14.2N + 22.6$	$q_a = 12.9N_{10}$
Case 1	60	200	150	115
Case 2	25	100	65	35
Case 3	15	70	40	15
Case 4	15	100	65	35
Case 5	15	100	65	35
Case 6	20	100	65	35
Case 7	15	100	65	35
Case 8	15	105	70	40
Case 9	20	105	70	40

**Figure 5.** Comparison of different SPT and DCPT correlations

4. Results and Discussions

The developed correlation between SPT and DCPT values for local sabkha soils suggest that a strong relationship exists between both parameters. Using regression analysis, an expression of $N_{30} = 0.4658N_{10} - 0.0807$ was obtained with an associated coefficient of determination of $R^2 = 0.9392$. The correlation obtained in this study is also plotted with the existing SPT and DCPT correlations by [22], [20] and [23] for data of Site 4 and results are provided in Figure 5. The existing correlations are not matching well with the one developed in the current study. The reasons could be that the correlation by [22] is based on N_{70} whereas the one suggested in this study is based on N_{60} values. [20] developed the correlation for clayey sands whereas sabkha soils tested in this study are sandy in nature. Another notable reason for this discrepancy could also be the cementation among the soil particles of sabkha soils, resulting from the

precipitation of salts such as Aragonite, Calcite, Gypsum and sodium chloride etc. in sabkha layers (Figure 2). The precipitation of salts occurs as a result of repeated cycles of capillary rise and evaporation in the soil layers, driven by fluctuations in ambient temperature, humidity, precipitation, and tidal variations. This cementation has resulted in higher SPT and DCPT values as compared to other correlations presented in literature where no such cementation is reported in the tested soils.

The allowable bearing capacities by Burland & Burbidge method (SPT based method) were two (02) to seven (07) times lower than the ones given by DCPT based methods (Table 3). The SPT based bearing capacity methods are well developed over time and account for different factors in their formulation like stress and strain influence factors, over consolidation ratios, correction factors to account for creep, shape factors, loading conditions etc. whereas most of these factors are ignored in DCPT based methods. Therefore,

it is more suitable to use SPT based methods for bearing capacity evaluation rather than DCPT based methods due to their inherent limitations and simplicity in formulation.

5. Conclusions

Sabkha areas are generally inaccessible to drilling rigs due to their low shear strength and high collapsibility, which significantly limits the acquisition of Standard Penetration Test (SPT) data. However, the Dynamic Cone Penetration Test (DCPT) offers a practical alternative in such environments, as its portable and lightweight nature makes it well-suited for use in these challenging ground conditions.

In this study, SPT and DCPT tests are carried out side by side on several sabkha sites and a site-specific correlation ($N_{30} = 0.4658 \times N_{10} - 0.0807$) is developed between SPT and DCPT for Sabkha soils. Since many geotechnical soil parameters are commonly correlated with SPT results, and numerous SPT-based empirical relationships are available in the literature for deriving foundation design parameters, the established correlation between SPT and DCPT for local Sabkha soils can be effectively used to convert DCPT values into equivalent SPT values. This facilitates the application of the extensive database of SPT-based correlations for Sabkha soils where only DCPT data can be collected.

6. Recommendations

Due to the inherent limitations and simplified assumptions in DCPT-based correlations for estimating allowable bearing capacity, it is recommended to use SPT-based methods for bearing capacity evaluation. This can be achieved by first converting DCPT values into equivalent SPT values using the established correlations, and then applying SPT-based approaches for bearing capacity evaluation.

REFERENCES

- [1] Abduljauwad, S.N., Al-Amoudi, O.S.B. & El-Naggar, Z.R. (1994). On the Geotechnical Problems of Sabkha Evaluation of Grain Size Distribution. Proc., 2nd Geotechnical Engineering Conference, Cairo University, Cairo, Egypt, Vol. I, pp. 2-20.
- [2] Aiban, S.A., Al-Ahmadi, H.M., Asi, I.M., Siddique, Z.U., & Al-Amoudi, O.S.B. (2006). Effect of Geotextile and Cement on the Performance of Sabkha Subgrade. Journal of Building and Environment, Vol. 41, 2006, pp. 807-820.
- [3] Amin, A. (2004). Comparative study of the geotechnical properties of the coastal Sabkhas of Saudi Arabia and their hazardous effects. Bulletin of Engineering Geology and the Environment, 63(4), 309-314.
- [4] Ampadu, S.I.K., & Dzitse-Awuku, D. (2009). Model tests for bearing capacity in a lateritic soil and implications for the use of the dynamic cone penetrometer. Proc. 17th Int. Conf. on Soil Mechanics and Geotechnical Engineering: 332-335.
- [5] Arifuzzaman, M., Habib, M. A., Al-Turki, M. K., Khan, M. I., & Ali, M. M. (2016). Improvement and characterization of Sabkha soil of Saudi Arabia: a review. Jurnal Teknologi, 78(6).
- [6] Akili, W. & Torrance, J.K. (1981). The Development and Geotechnical Problems of Sabkha, with Preliminary Results on Static Cone Penetration Resistance of Cemented Sands," Quarterly Journal of Engineering Geology, Vol. 14, pp. 59-73.
- [7] Al-Amoudi, O.S.B., Abduljauwad, S.N., El-Naggar, Z.R., & Rasheeduzzafar (1992). Response of Sabkha to Laboratory Tests: A Case Study. Engineering Geology, Vol. 33, No. 2, pp. 111-125.
- [8] Al-Amoudi, O.S.B. (1994). Chemical Stabilization of Sabkha Soils at High Moisture Contents. Engineering Geology, Vol. 36, Nos. 3/4, pp. 279-291.
- [9] Al-Amoudi, O.S.B. & Abduljauwad, S.N. (1995). Strength Characteristics of Sabkha Soils. *Geotechnical Engineering Journal*, Vol. 26, No. 1, pp. 733-92.
- [10] Al-Amoudi, O.S.B. (1999). Soil Stabilization and Durability of Reinforced Concrete in Sabkha Environments. Journal of King Abdulaziz University: Engineering Sciences, Special Issue on: Fourth Saudi Engineering Conference, pp. 53-72.
- [11] Al-Shamrani, M.A., & Dowian, A.W. (1997). Preloading for Reduction of Compressibility Characteristics of Sabkha Soil Profiles. Engineering Geology, Vol. 48, pp. 19-41.
- [12] Al-Amoudi, O. S. B. (2008). Testing and stabilization of saline Sabkha soils: a review. 6th International Conference on Case Studies in Geotechnical Engineering, Arlington, VA, 11-16 (2008).
- [13] Bergdahl, U. & Eriksson, U. (1983). Soil properties with Penetration Tests – A Literature study. Swedish Geotechnical Institute. Linköping, Sweden, Report No. 22.
- [14] Butcher, A. P., McElmml, K., & Powell, J. J. M. (1996). Dynamic probing and its use in clay soils. Proceedings of the international conference on Advances in site investigation practice, (Craig C. (eds)). Thomas Telford, London: 383-395.
- [15] BS EN ISO 22476-2:2005/A1 (2011). Geotechnical investigation and testing – Field testing – part 2: Dynamic probing, British Standards Institution.
- [16] Cearn, P. J., & Mckenzie, A. (1988). Application of dynamic cone penetrometer testing in East Anglia. Proceeding of Geotechnology Conference on Penetration Testing In the UK, Thomas Telford, London: 123-127.
- [17] DIN 4094 part 1 (1974). Dynamic and Statics penetrometers, Dimensions of apparatus and method of operation. Deutsches Institut für Normung e. V. Berlin.
- [18] DIN 4094 part 2 (1980). Dynamic and Statics penetrometers, Application and evaluation. Deutsches Institut für Normung e. V. Berlin.
- [19] Hakami, A., Ghrefat, H., Elwaheidi, M., Galmed, M., & Yahya, M. A. (2022). Assessment of the corrosivity of the Southern Red Sea coastal Sabkha soil: An integrated mineralogical, geochemical, and GIS approach. Environmental Earth Sciences, 81(225).
- [20] Ibrahim, M., & Nyaoro, D.L. (2011). Correlations of DCPT and SPT for analysis and design of foundations, Proc. 15th African Regional Conf. on Soil Mechanics and Geotechnical

- Engineering, July 18 to 21, 2011, Maputo, Mozambique, IOS press, Amsterdam, Netherlands: 632-637.
- [21] Kleyn, E.G., & Savage, P.F. (1982). The application of the pavement DCP to determine the bearing properties and performance of road pavements. Proc. Int. Symp. on Bearing Capacity of Roads and Airfields, June 23-25, 1982, Trondheim, Norway: 238-246.
 - [22] Opuni, K.O., Nyako, S.O., Ofosu, B., Mensah, F.A., & Sarpong, K. (2017). Correlations of SPT and DCPT data for sandy soils in Ghana. *Lowland Technology International* 2017; 19 (2): 145-150.
 - [23] Ligwanda, M.I., Larsson, S., & Nyaoro, D.L. (2015). Correlations of SPT, CPT and DPL data for sandy soil in Tanzania. *Geotechnical and Geological Engineering*, 33 (5): 1221-1233.
 - [24] Livneh, M. (1987). Validation of correlations between a number of penetration tests and in situ California bearing ratio tests. *Transport Research Record* 1219, Transportation Research Board, Washington: 56-67.
 - [25] Sanglerat, G. (1972). *Developments in Geotechnical engineering: The penetrometer and soil exploration*. Elsevier Publishing Co., Amsterdam, The Netherlands: pp 464.
 - [26] Saudi Building Code, SBC-303 (2018). *Structural-soils and foundations*. Saudi Building Code National Committee, Building Standards, Saudi Arabia.
 - [27] Scala, A.J., 1 (1956). Simple methods of flexible pavement design using cone penetrometers, 11 (2): 33-44.
 - [28] Spagnoli, G. (2007). An empirical correlation between different dynamic penetrometers. *J. Geotechnical Engineering* 12 (BundleC). Assessable from <http://www.ejge.com/2007/Ppr0729/Abs0729.htm>.
 - [29] Spyropoulos, E., Touma, J.F., Ahmed, H.R., & Waheed, M.A. (2021). CPT and SPT as Complementary Tests for the Formulation of Geotechnical Design Profiles. *GeoChina 2021, SUCI*, pp. 83-92, 2021. https://doi.org/10.1007/978-3-030-79672-3_6.